

# Optimal Sensor Placement in a Partitioned Water Distribution Network for the Water Protection from Contamination <sup>†</sup>

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**Abstract:** Water network protection from accidental and intentional contamination is one of the most critical issues for preserving the citizen health. Recently, some techniques have been proposed in the literature to define the optimal sensor placement. On the other hand, through the definition of permanent DMAs (District Meter Areas), water network partitioning allows significant reduction in the number of exposed users through the full isolation of DMA. In this paper, the optimal sensor placement is coupled with water network partitioning in order to define the best location of isolation valves and control stations, to be closed and installed respectively. The proposed procedure is based on different procedures, and it was tested on a real water network, showing that it is possible both to mitigate the impact of a water contamination and simplify the sensor placement through the water network partitioning.

**Keywords:** water protection; sensor placement; water network partitioning; water contamination

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## 1. Introduction

The “divide and conquer” concept has recently been gaining attention in the management of water distribution networks (WDNs), since dividing large-scale networks into smaller and manageable subsystems (District Metered Areas, DMAs), offers advantages for the monitoring and control of consumption and leakage. In the scientific literature, numerous works were dedicated to the design of DMAs, based on the application of graph and spectral theory algorithm [1–5], or based on the concept of modularity function [6–9].

In this framework, one of the main research issues lies in determining the optimal location of sensors, able to detect the most common water parameters and, as a result, to monitor the WDN by identifying possible contaminations [10–13]. This paper explores the benefits of network partitioning for the optimal placement of quality sensors for water distribution network (WDN) protection from contaminations. The global aim is to show how the water network partitioning improves the protection of the WDNs against a possible contamination, both accidental and intentional. The proposed methodology was tested on a real WDN, showing that the partitioning successfully mitigates the impact of contaminations in terms of affected population, thanks to the reduction in the total number of water paths in the WDN.

## 2. Materials and Methods

The methodology is based on the combination of two main procedures: the former procedure enables WDN partitioning by clustering network nodes for district metered area (DMA) identification, and by separating the DMAs through gate valve closure or flow meter installation at each boundary pipe; the latter procedure is for the optimal placement of quality sensors in undivided or partitioned WDNs.

### 2.1. Network Partitioning

WDN partitioning is carried out in two main phases:

- *clustering*, in which the optimal shape and size of the clusters are defined by minimizing the number of edge-cuts (boundary pipes) and by simultaneously balancing the number of nodes of each cluster, and
- *dividing*, in which clusters are separated from each other by closing isolation valves at some boundary pipes and installing flow meters at the remaining boundary pipes.

In this work, the clustering layout is obtained exploiting the properties of the normalized Laplacian matrix  $L = D - A$ , in which  $D$  is the diagonal matrix containing the node degree  $k_i$  of each node, and  $A$  is the adjacency matrix, which elements  $a_{ij} = a_{ji} = 1$  if nodes  $n_i$  and  $n_j$  are connected by a pipe,  $a_{ij} = a_{ji} = 0$  otherwise. On the Laplacian matrix the spectral clustering algorithm was applied, for which the main steps in the case of a WDN are described in [14]. The graph of the WDN was considered un-weighted (every connection between the nodes has the same importance). The clustering phase provides the optimal cluster layout and the edge-cut set  $N_{ec}$ .

Regarding the dividing phase, the choice must be made whether either a gate valve must be closed, or a flow meter must be installed in the generic boundary pipe, in a way that, the sum of closed gate valves  $N_{gv}$  and installed flow meters  $N_{fm}$  must be equal to  $N_{ec}$ . Closing gate valves could reduce the service pressure, so it is important to guarantee that the service pressure in each point was higher than the desired threshold value  $h_{des}$ . In this work, the trade-off between leakage and WDN reliability was explored through the bi-objective optimization, performed through the NSGAI genetic algorithm [15]. The first objective function  $f_1$  to minimize was the daily leakage:

$$f_1 = V_l, \quad (1)$$

The second objective function  $f_2$  relates to the global resilience failure index  $GRF$  index proposed by [16], which is the sum of the resilience ( $I_r$ ) and failure ( $I_f$ ) indices evaluated at the generic instant of WDN operation:

$$GRF = I_r + I_f = \frac{\max(\mathbf{q}_{user}^T \mathbf{H} - \mathbf{d}^T \mathbf{H}_{des}, 0)}{\mathbf{Q}_0^T \mathbf{H}_0 + \mathbf{d}^T \mathbf{H}_{des}} + \frac{\min(\mathbf{q}_{user}^T \mathbf{H} - \mathbf{d}^T \mathbf{H}_{des}, 0)}{\mathbf{d}^T \mathbf{H}_{des}} \quad (2)$$

where  $d$  and  $q_{user}$  are the vectors of nodal demands and water discharges delivered to users, respectively, at WDN demanding nodes. In this work,  $q_{user}$  was evaluated as a function of  $d$  and pressure head  $h$  at each node through the pressure driven formula of [17].  $H$  and  $H_0$  are the vectors of nodal heads at demanding nodes and sources, respectively.  $H_{des}$  is the vector of desired nodal heads, which are the sum of nodal elevations and desired pressure heads  $h_{des}$ . Finally,  $Q_0$  is the vector of the water discharges leaving the sources. The  $GRF$  index has the advantage of being always within range  $[-1, 1]$ . Higher values of  $GRF$  indicate higher power delivered to WDN users and, therefore, higher service pressure. The objective function  $f_2$  was calculated with the relationship suggested by [16],  $f_2 = \text{median}(GRF)$ .

The Pareto front of optimal solutions will be re-evaluated also in terms of  $N_{fm}$  (as a surrogate for the partitioning cost) and demand satisfaction rate  $I_{ds}$  (that represents the effectiveness of the service to WDN users); in particular, it can be calculated as the ratio between delivered water volume  $w_d$  ( $m^3$ ) and WDN demand  $w_{tot}$  ( $m^3$ ).

## 2.2. Optimal Sensor Placement

Let a set  $S$  of significant contamination events, each of which featuring a certain location, starting time, duration and total mass, be defined. In this context, sensor placement can be formulated as a bi-objective optimization problem [13], in which the first objective function  $f_3$  is the number  $N_{sens}$  of installed sensors (as a surrogate for the installation cost for the WDN protection)

$$f_3 = N_{sens}, \quad (3)$$

and the second objective function  $f_4$  is related to the contaminated population  $pop_r$  before the first detection of the generic  $r$ -th contamination event. This corresponds to the sum of the inhabitants served by the contaminated nodes and can be evaluated using the EPANET quality solver [18], considering an unreactive contaminant as assumption of the first attempt.

The time interval  $\Delta t_{react}$  (the time for a warning to interrupt network service) is set to 0 hereinafter for simplifying purposes but can be set to other values without loss of validity of the whole methodology. The function  $f_4$  is calculated as the weighted average value  $pop$  of  $pop_r$ , that is:

$$f_4 = pop = \frac{\sum_{r=1}^S w_r pop_r}{\sum_{r=1}^S w_r}, \quad (4)$$

where  $w_r$  is a weight coefficient associated with the generic contamination event.

Functions  $f_3$  and  $f_4$  are minimized simultaneously through the NSGAI genetic algorithm [15]. In the population individuals of NSGAI, the number of genes is equal to the number of network nodes where sensors can be installed. Each gene can take on the two possible values 0 and 1, which stand for absence and presence of the sensor in the node associated with the gene, respectively.

## 3. Case Study

The methodology described above was tested on the WDN of Parete [19], which is a small town located in a densely populated area to the south of Caserta (Italy), with population of 11,150 inhabitants. This WDN has 182 demanding nodes (with ground elevations ranging from 53 m a.s.l. to 79 m a.s.l.), 282 pipes and 2 sources with fixed head of 110 m a.s.l. (Figure 1).

A uniform desired pressure head  $h_{des} = 9 + 10 = 19$  m was assumed for the demanding nodes (9 m is the height of the average building in Parete while 10 m is the surplus of head as prescribed by the Italian guidelines).

Reference was made to the day of maximum consumption in the year with an average value of the node water demand of 36.3 L/s. The leakage volume of the networks in the day of maximum consumption adds up to 930 m<sup>3</sup> (about 23% of the total outflow from the sources).

The water quantity simulations were run for one day of WDN operation. For the construction of the set  $S$  of contamination events, all the 182 demanding nodes were considered as potential locations for contaminant injection, 24 possible contamination times in the day (hour 0, 1, 2, ..., 22, 23), a single value of the mass injection rate equal to 350 gr/min, and a single value of the injection duration equal to 60 min were assumed.

The values for mass injection and duration were sampled from those proposed by [11]. According to the procedure of [13], and considering the previous assumptions, the total number  $S$  of contamination events was  $182 \times 24 \times 1 \times 1 = 4368$ . The weight of the generic event  $w_r$  was set to 1 to give identical relevance to all of them. The water quality simulations were run for 3 days of WDN operation to make sure that even contaminants injected close to the sources at the last instant of the first day had enough time to leave the network.

After the definition of the optimal partitioning, two cases were analysed, in particular, Case 1, with Optimal sensor placement on the original un-partitioned WDN, and Case 2, with Optimal sensor placement on partitioned WDN. In all the applications, the NSGAI was applied with a population of 300 individuals and a total number of 300 generations.

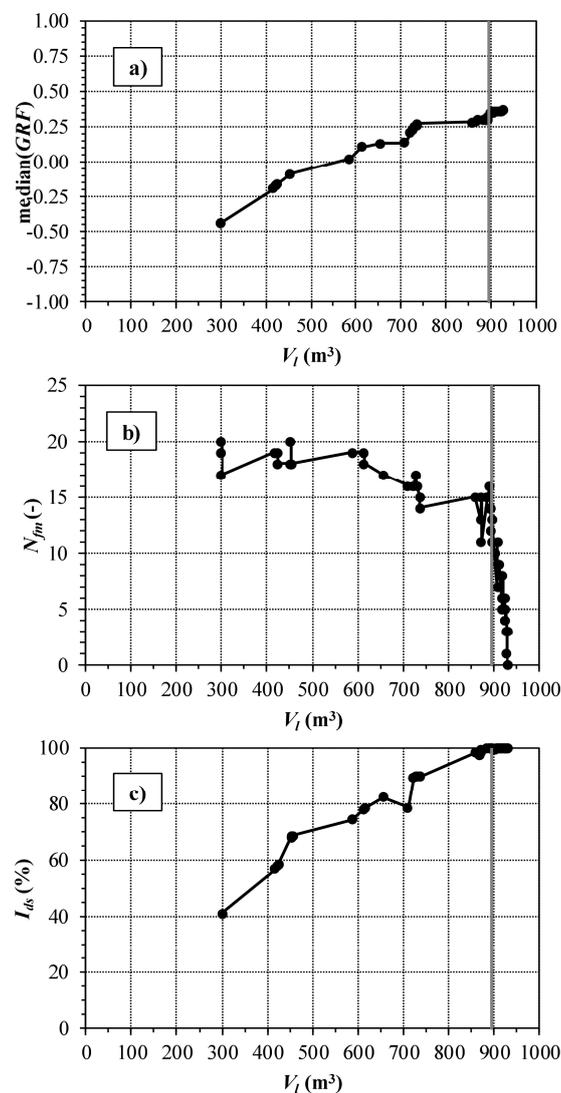
### 4. Results and Discussion

Following, the results were presented for both Cases, comparing them in terms of exposed population. The water network partitioning leads to produce 5 DMAs; in Table 1, the number of nodes obtained in each DMA is reported, as well as the number  $N_{ec}$  of boundary pipes.

**Table 1.** Number of nodes in the various DMAs and  $N_{ec}$  of boundary pipes for the partitioning of the Parete WDN in 5 DMAs.

DMA1	DMA2	DMA3	DMA4	DMA5	$N_{ec}$
20	35	39	41	49	21

For the dividing phase, the optimization through NSGAI1 yielded the Pareto front reported in Figure 1a, showing, as expected, growing values of median ( $GRF$ ) with  $V_l$  growing, since both variables are growing functions of the service pressure in the WDN.

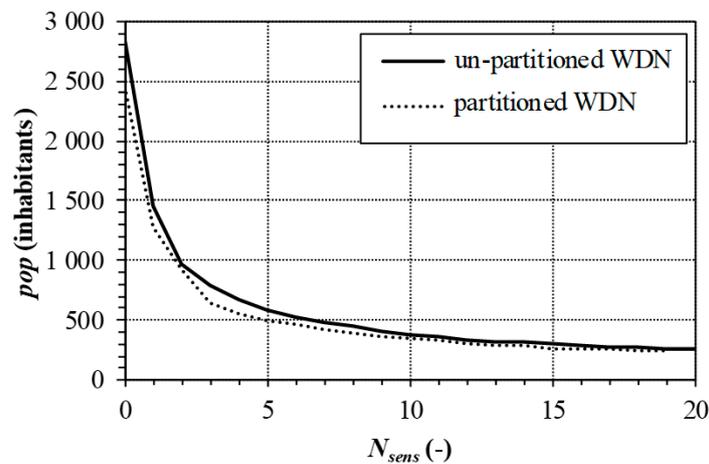


**Figure 1.** Dividing phase for the Parete WDN. Pareto front of optimal solutions in the trade-off between daily median  $GRF$  index and leakage volume  $V_l$  (a), re-evaluated solutions in terms of number of installed flow meters  $N_{fm}$  (b) and of demand satisfaction rate  $I_{ds}$  (c). In all graphs, the selected solution is highlighted with a grey vertical line.

Graphs (b) and (c) report the number  $N_{fm}$  of installed flow meters and the demand satisfaction rate  $I_{ds}$ , respectively, re-evaluated from the Pareto front and plotted against  $V_l$ . Globally, graph (b)

highlights that the higher values of  $N_{fm}$  tend to be associated with the lower values of  $V_l$ . This is because  $V_l$  tends to grow when fewer gate valves are closed (and then more numerous flow meters are installed) in the boundary pipes. Finally, graph c) shows that  $I_{ds}$  tend to grow with  $V_l$  increasing, since both variables are increasing functions of the service pressure. From the graphs in Figure 1, the solution with the lower value of  $N_{fm}$  ( $=8$ ), higher number of closed valves  $N_{gv}$  ( $=13$ ), which ensures  $I_{ds} = 100\%$ , was finally chosen, which enables also reducing leakage around 3,7% (from 930 m<sup>3</sup> for the un-partitioned layout to 895 m<sup>3</sup>). The corresponding median (*GRF*) is equal to 0.32, very close to the value of 0.36 for the un-partitioned network.

After the definition of the optimal partitioning, the optimal sensor placement was carried out on the original un-partitioned network and on the partitioned one. The Pareto fronts for the optimal sensor placement are shown in Figure 2; the results for the original un-partitioned WDN are represented as solid black lines, while for the partitioned network they are reported as dotted black line. As expected, for both Cases, the fronts show decreasing values of *pop* as  $N_{sens}$  increases up to 20. However, for higher values of  $N_{sens}$ , the additional benefit of a further sensor installed in the network tends to decrease, as already pointed out by [13].



**Figure 2.** Pareto fronts of optimal sensor placement solutions in the trade-off between  $N_{sens}$  and *pop*, obtained for the original un-partitioned WDN (Case 1), and for the partitioned WDN (Case 2).

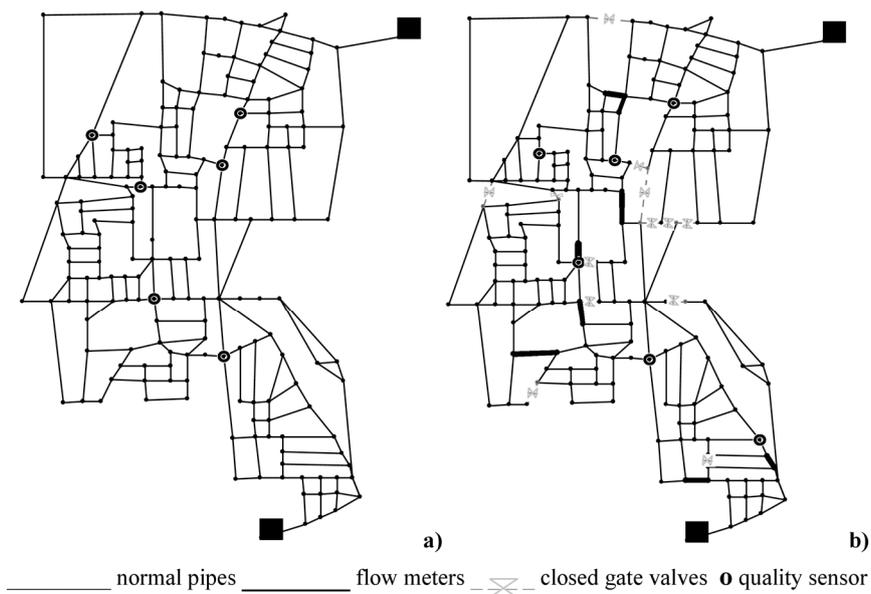
The comparison points out better solutions for Case 2 (partitioned network), above all for low values of  $N_{sens}$ , as shown by the values of *pop* in Figure 2 and by the values of the percentage difference calculated as (value of *pop* for Case 1 – value of *pop* for Case 2)/value of *pop* for Case 1, in Table 2. In particular, for  $N_{sens} = 6$ , the contaminated population for the un-partitioned network is *pop* = 514 (a reduction by 81.7% compared to *pop*=2806 for  $N_{sens} = 0$  in the un-partitioned network), while for the partitioned network *pop* = 457 (a reduction by 83.7% compared to *pop*=2806 for  $N_{sens} = 0$  in the un-partitioned network). This is because the partitioning *per se* causes a reduction in the total number of water paths in the WDN and, therefore, in the contaminated population (highlighted for a number of sensors  $N_{sens} = 0$ , where the difference of the contaminated population is around 11.7%). Therefore, the optimal combination of  $N_{sens}$  sensors in a partitioned WDN always outperforms the corresponding one in the original WDN.

In this regard, for the partitioned network, installing the same number of sensors in the WDN of Parete leads to a reduction of the contaminated population, with respect to the un-partitioned network, ranging from 7.3% (corresponding to  $N_{sens} = 2$ ) to 17.9% (corresponding to  $N_{sens} = 4$ ). In particular, the partitioning does not only reduce *per se* the contaminated population but also improves the efficiency of the sensor station systems. This means that, the water network partitioning is a valid strategy to better manage the WDNs and simultaneously to guarantee the water network protection from contamination (both accidental and intentional), confirming its dual-use.

**Table 2.** Simulation results in terms of exposed population for the two Cases for the Parete WDN considering the installation of  $N_{sens}$  up to 6.

$N_{sens}$ (-)	(Case 1)		(Case 2)		Difference (%)
	Pop	Reduction (%)	Pop	Reduction (%)	Case 1-Case 2
0	2806	0.0	2479	11.7	11.7
1	1438	48.8	1265	54.9	12.1
2	982	65.1	911	67.5	7.3
3	789	71.9	648	76.9	17.9
4	667	76.2	554	80.3	16.9
5	589	79.0	504	82.1	14.4
6	514	81.7	457	83.7	11.1

The layouts in Figure 3 show the optimal location of 6 sensors obtained in the original un-partitioned WDN, and in the partitioned WDN. It is clear that, for the two layout of the Parete WDN (un-partitioned and partitioned), 5 of the 6 sensors are located about in the same areas. It suggests that, it is possible to define “most influential” nodes in a WDN regardless the operational conditions, the monitoring of which, ensures an efficient monitoring of the system. This crucial aspect will be further investigated in order to establish topological criteria able to individuate *a priori* these points.



**Figure 3.** Optimal location of 6 sensors in (a) original un-partitioned WDN; (b) partitioned WDN.

### 5. Conclusions

In this work, a methodology based on WDN partitioning and on optimal placement of quality sensors was set up to investigate the benefits of “divide and conquer” technique for the protection of WDNs from contamination events. The applications concerned a real Italian WDN, which was first partitioned in 5 DMAs separated each other by either closing gate valves or installing flow meters at boundary pipes. Optimal sensor placement solutions were searched for on the original undivided WDN and on the partitioned layout, in the trade-off between number of installed sensors and affected population for an assigned set of contamination events. The results showed that, for a given number of installed sensors, the monitoring stations installed in the partitioned layouts offer better protection from contamination. Future work will be dedicated to the issues of DMA restoration after the generic contamination. This will be done with reference to specific real contaminants, while abandoning the simplifying assumption of un-reactive and conservative contaminant adopted so far, in an attempt to make the results more realistic.

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